Trapping of sediment in tidal estuaries

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<u>Chesapeake Bay</u>: Length: 315 km Width: 5-56 km Av. depth: 8.5 m

German Wadden Sea:



Many estuaries exhibit an Estuarine Turbidity Maximum comprising fine, suspended muddy sediments.



1. Potomac 2. Chesapeake Bay 3. Delaware 4. Severn



Other example: Ems estuary.



Ems River at a glance

~ 12,600 km²

 $Q_{avg} \sim 70 \text{ m}^3/\text{s}$



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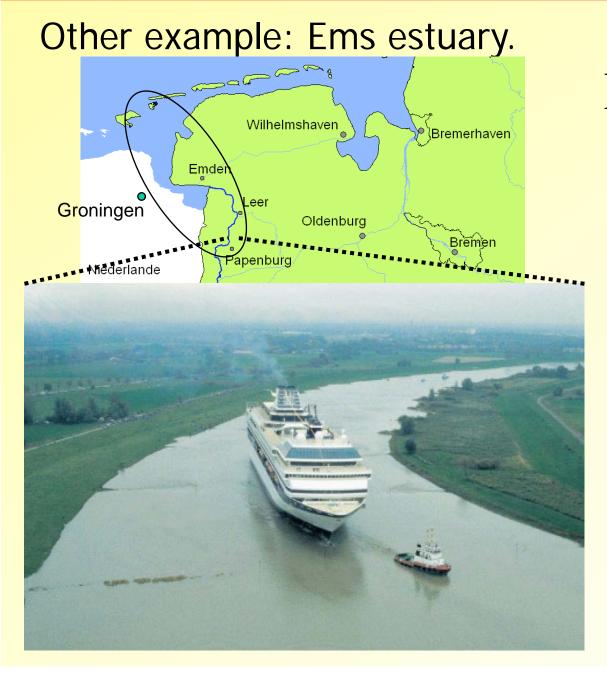
Q_{avg} ~ 70 m³/s

Our focus: Ems Estuary

Shipping most Important Industry

e.g. MeyerWerft

Large Implications for river And estuarine dynamics!



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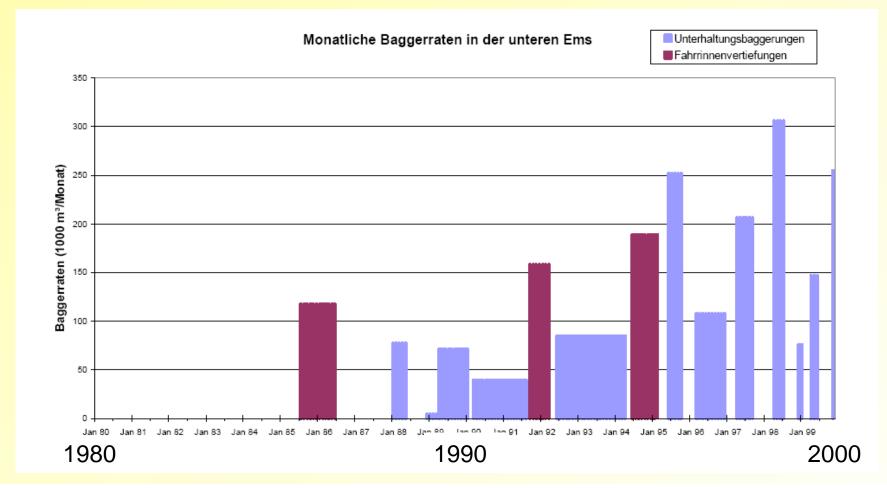
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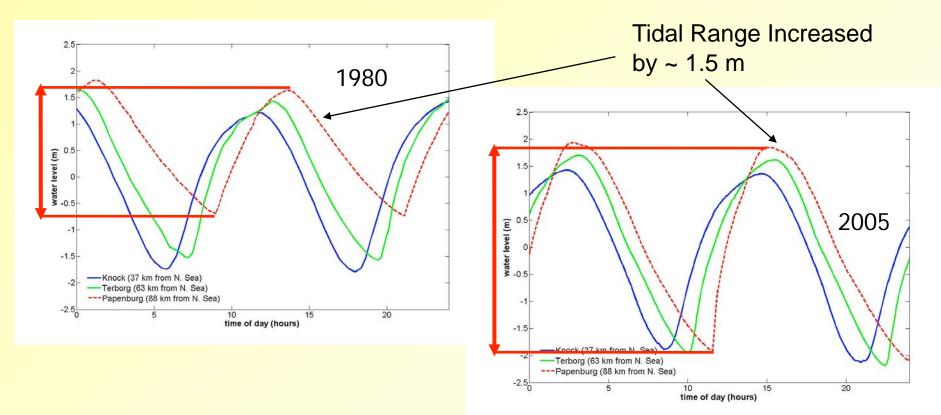
Increase in Dredging



Monthly dredging rates in the Ems between Emden and Pappenburg between 1980 and 2000, in units of 1000 m³/month. Adapted from Habermann, 2003.

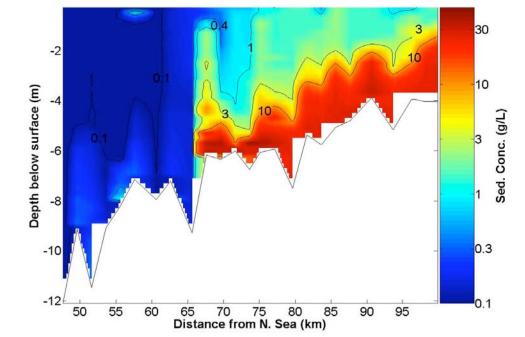
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Changes in horizontal velocity?





- Turbidity maximum has moved upstream;
- High turbidity zone now extended into the freshwater zone to Papenburg;

Research Questions

• Can the observed changes in the water motion be modelled and understood?

• Which mechanisms result in trapping of sediment in the Ems and what has changed over the years?

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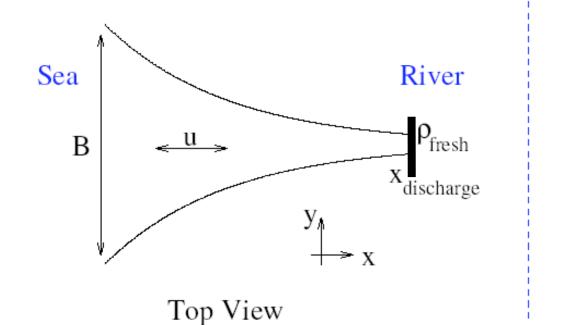
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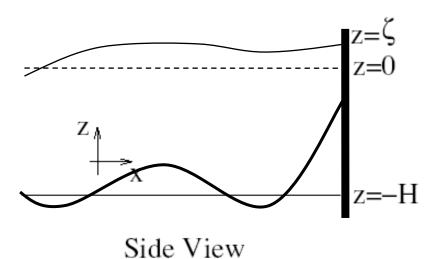
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Main Results

Essential ingredients

- Decrease of bed friction and vertical mixing and a deepening of the channel
- Along-estuary varying erosion coefficient (~ layer of fine sediment)
- Temporal settling lag effects + external overtides



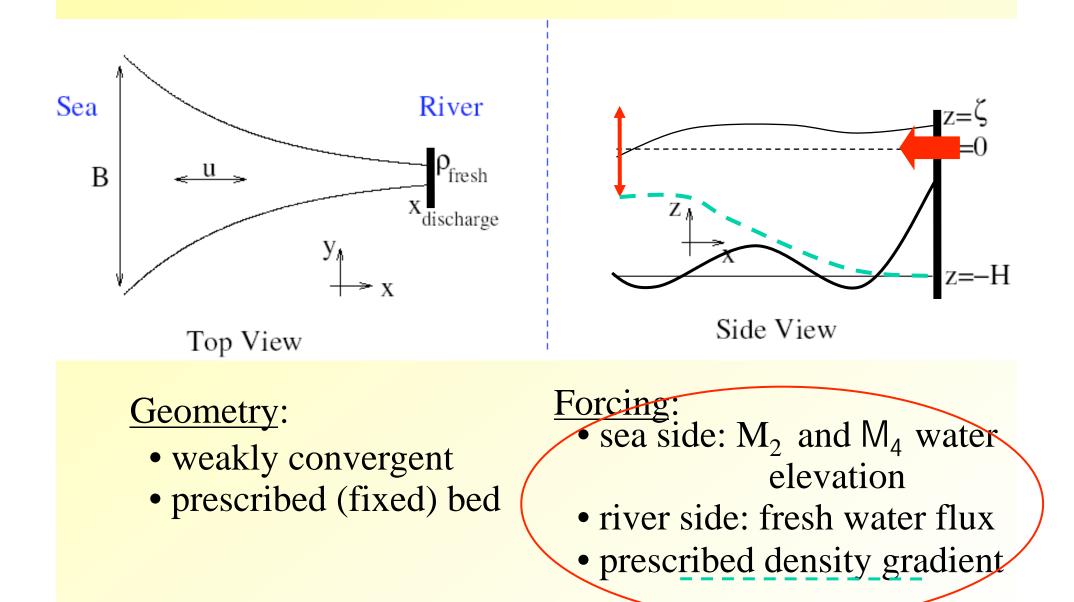


Geometry:

- weakly convergent
- prescribed (fixed) bed

Forcing:

- sea side: M₂ and M₄ water elevation
- river side: fresh water flux
- prescribed density gradient



 Water Motion: 2 DV (width averaged) shallow water equations (residual, M₂ and M₄ components)

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$$u_x + w_z - \frac{u}{L_b} = 0,$$

$$u_t + uu_x + wu_z + g\zeta_x - \frac{g\rho_x}{\rho_0}(z - \zeta) - (A_v u_z)_z = 0.$$

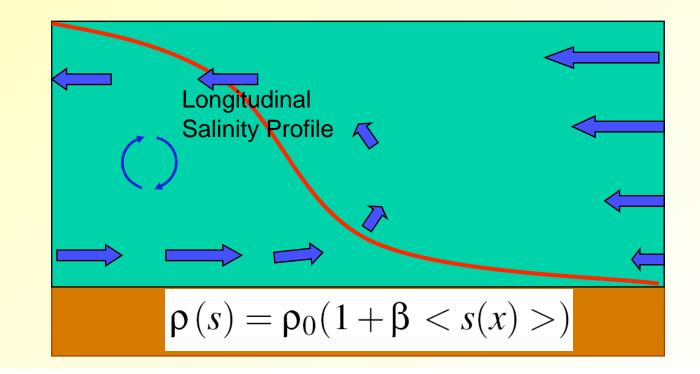
+ appropriate boundary conditions

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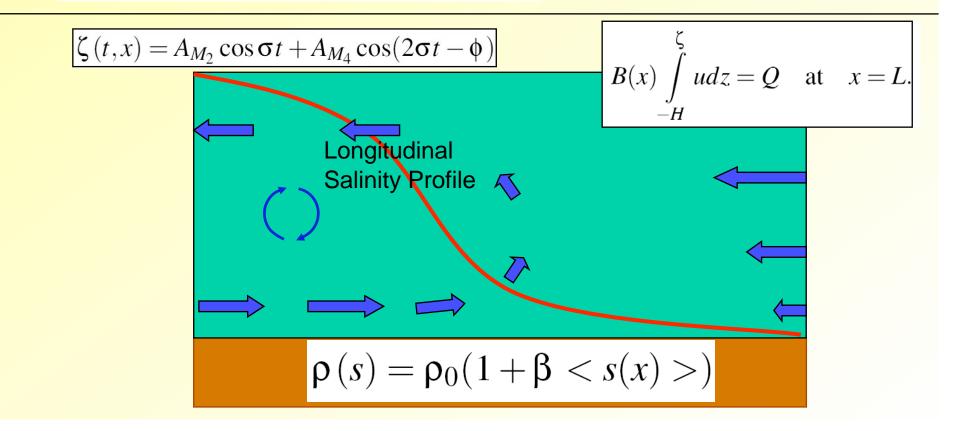


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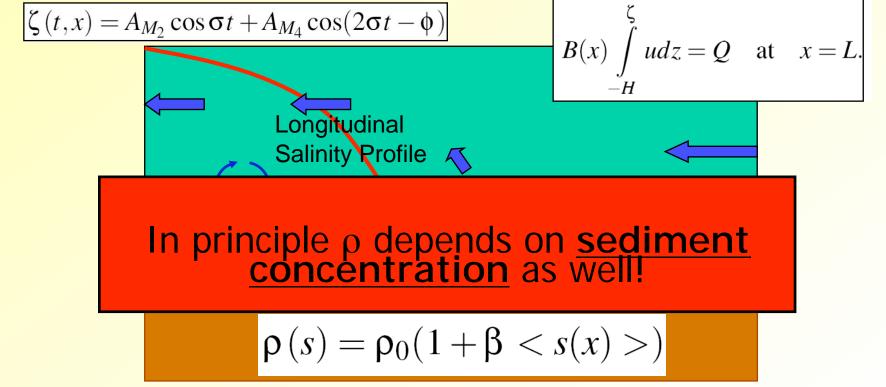


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 Water Motion: 2 DV (width averaged) shallow water equations(residual, M₂ and M₄ components)

- Suspended load transport:
- advection-diffusion equation
- deposition
- erosion ~ a(x) |u|

Erosion flux:
$$E_s \equiv -K_v \frac{\partial c}{\partial z} n_z - K_h \frac{\partial c}{\partial x} n_x = w_s c_*$$

Deposition flux: $D = w_s c n_z$

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Deposition flux: $D = w_s c n_z$ $c_*(t,x) = \rho_s \frac{|\tau_b(t,x)|}{\rho_0 g' d_s} a(x)$

 Water Motion: 2 DV (width averaged) shallow water equations(residual, M_2 and M_4 components)

- Suspended load transport:
 advection-diffusion equation
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- Diagnostic in density
- Bed evolution:

(1-p)
$$\rho_{\rm s} z_{\rm b} = - \boldsymbol{\nabla} \cdot \boldsymbol{q}_{\rm s}$$



Convergence: increase of $z_{\rm b}$ Divergence: decrease of $z_{\rm b}$

 Water Motion: 2 DV (width averaged) shallow water equations(residual, M₂ and M₄ components)

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$$\delta_{\tau} z_{b} = - \nabla \cdot \mathbf{q}_{s}$$

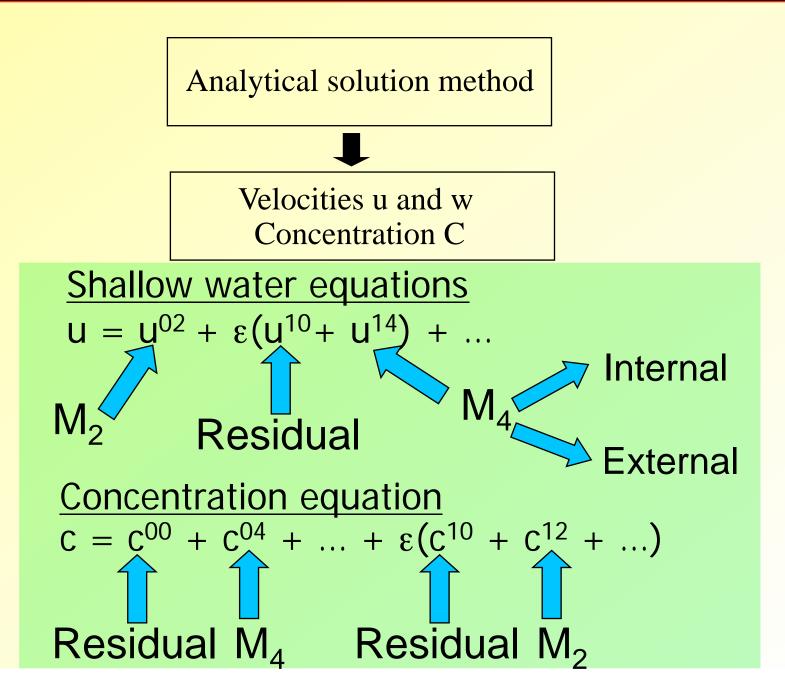
With the flux
$$\mathbf{q}_{s} = \left\langle \int_{-H}^{\zeta} (uc - K_{h}c_{x})dz \right\rangle$$

Solution Method (1)

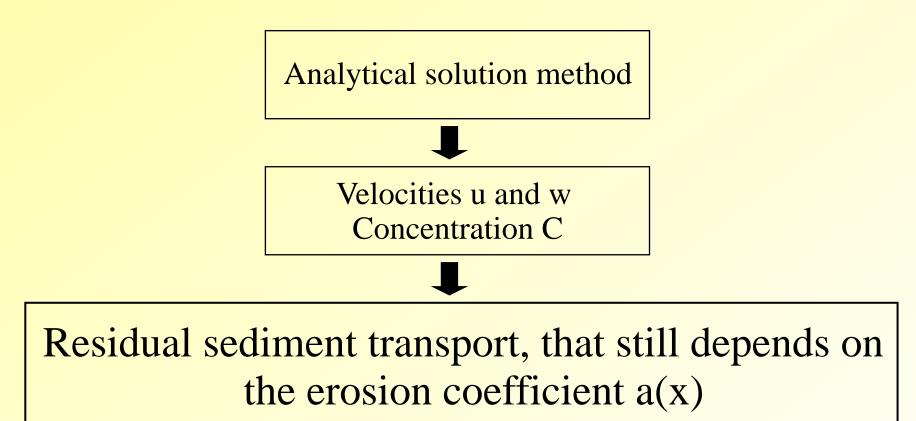
Analytical solution method

Perturbation approach: physical variables are expanded in power series of a small parameter $\epsilon = AM2/H$.

Solution Method (2)



Solution Method (3)



$$\int_{-H}^{0} (u^{10}c^{00} + \langle u^{02}c^{12} \rangle + \langle u^{14}c^{04} \rangle - K_h \langle c_x^{00} \rangle) dz + \langle \zeta^0 [u^0 c^0]_{z=0} \rangle$$

Solution Method (4)

Residual sediment transport, that still depends on the erosion coefficient a(x)

• Assume morphodynamic equilibrium : no residual sediment transport

 $T(x) a(x) + F(x) a_x(x) = 0$ With T(x): - < u > <C> : residual contribution $- < u_{M_2}C_{M_2}> : settling lag (M_2)$ $- < u_{M_4}C_{M_4}> : tidal asymmetry (M_4)$ diffusive contribution sum of all terms $- < K_hC> : settling lag$

Model Results (1)

Experiments

- Two years are considered: 1980 and 2005.
 Most parameters are obtained from observations

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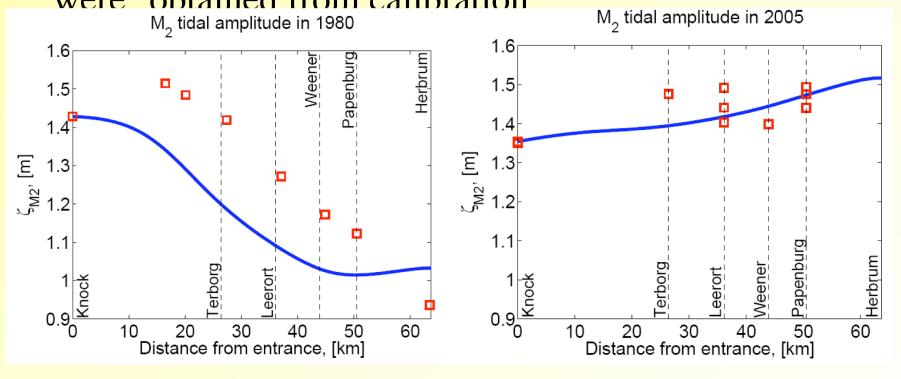
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Model Results (2)

Experiments

- Two years are considered: 1980 and 2005.
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 Only the vertical mixing and bottom friction were obtained from calibration



0.0124 m²/s 0.049 m/s

0.0187 m²/s 0.098 m/s Vertical Mixing Bottom Friction

Model Results (3)

Experiments

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 Only the vertical mixing and bottom friction were obtained from calibration
- Furthermore we choose: River outflow = $70 \text{ m}^3/\text{s}$ Settling velocity = 2 mm/s

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Using this information, we can solve for the unknown erosion coefficient:

 $T(x) a(x) + F(x) a_x(x) = 0$

Model Results (4)

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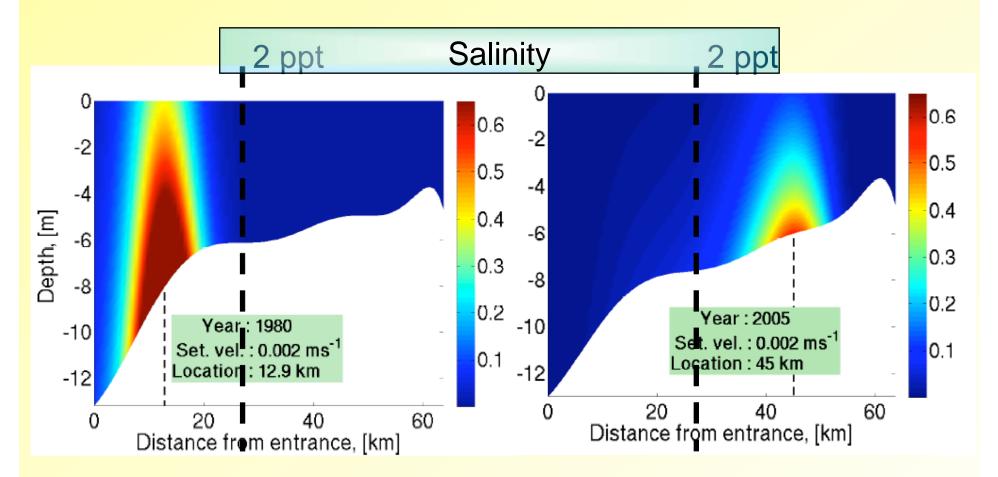
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This gives the sediment trapping in the estuary

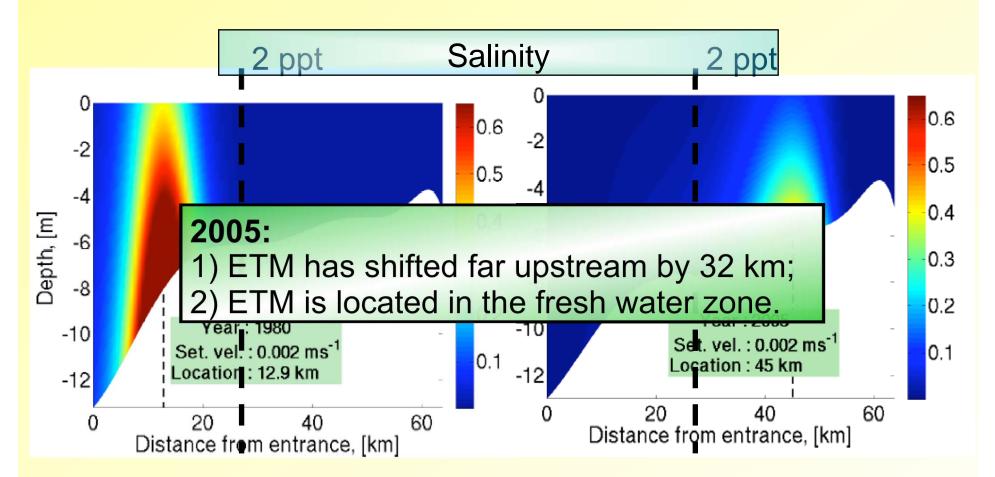
Model Results (5)



Model parameters:

- River discharge **70 m³/s**;
- Setting velocity 2 mm/s.

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Model Results (6)

Maximum of sediment concentration coincides with zeros of transport function T.

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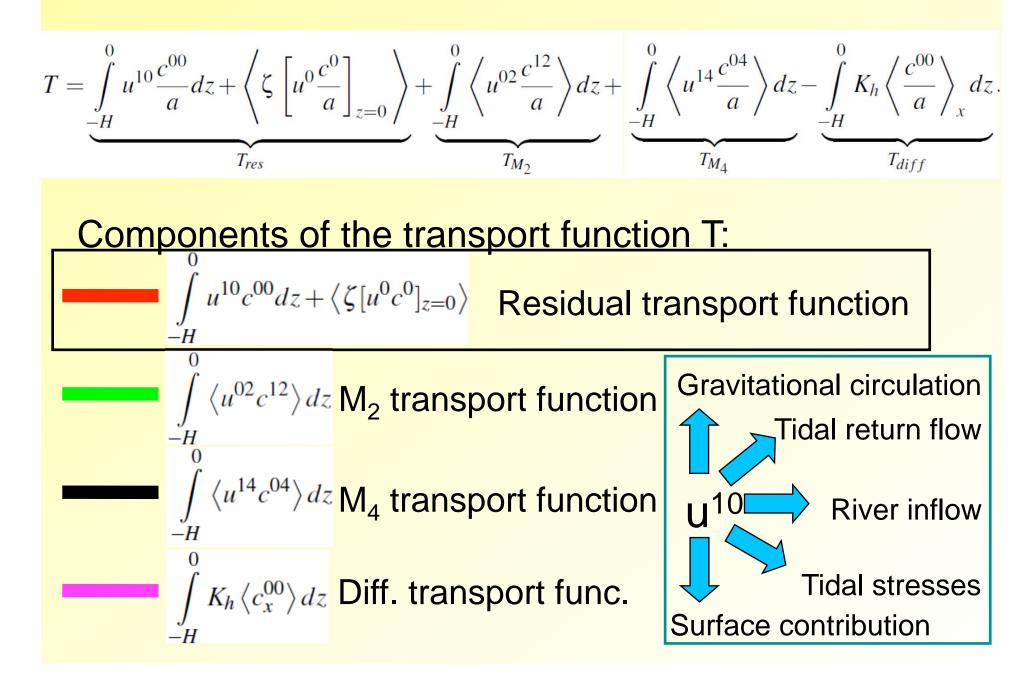
So to understand the changes in the trapping location of the sediment, we have to inspect the function T and its different contributions more carefully:

Model Results (7)

$$T = \underbrace{\int_{-H}^{0} u^{10} \frac{c^{00}}{a} dz}_{T_{res}} + \underbrace{\zeta \left[u^0 \frac{c^0}{a} \right]_{z=0}}_{T_{M_2}} + \underbrace{\int_{-H}^{0} \left\langle u^{02} \frac{c^{12}}{a} \right\rangle dz}_{T_{M_2}} + \underbrace{\int_{-H}^{0} \left\langle u^{14} \frac{c^{04}}{a} \right\rangle dz}_{T_{M_4}} - \underbrace{\int_{-H}^{0} K_h \left\langle \frac{c^{00}}{a} \right\rangle_x dz}_{T_{diff}}$$

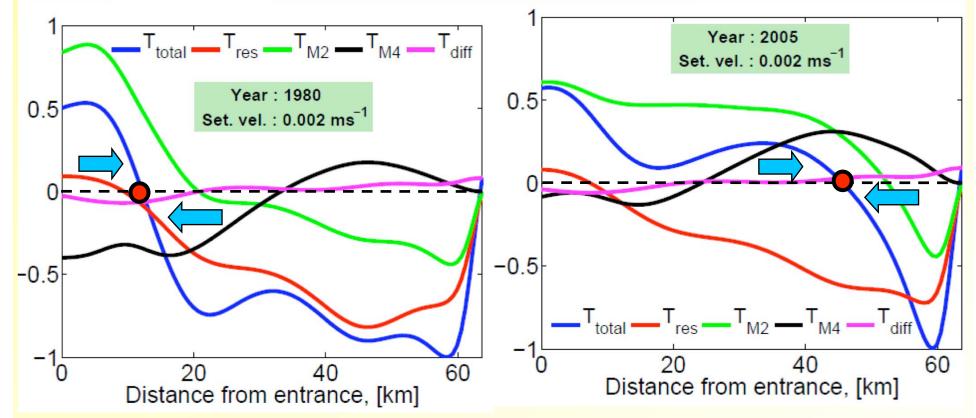
Components of the transport function T: $\int u^{10}c^{00}dz + \langle \zeta[u^0c^0]_{z=0} \rangle$ Residual transport function $\int_{-H}^{0} \langle u^{02}c^{12} \rangle dz$ M₂ transport function $\int_{-H}^{0} \langle u^{14}c^{04} \rangle dz$ M₄ transport function $\int_{-H}^{0} K_h \langle c_x^{00} \rangle dz$ Diff. transport func.

Model Results (3)



Model Results (7)

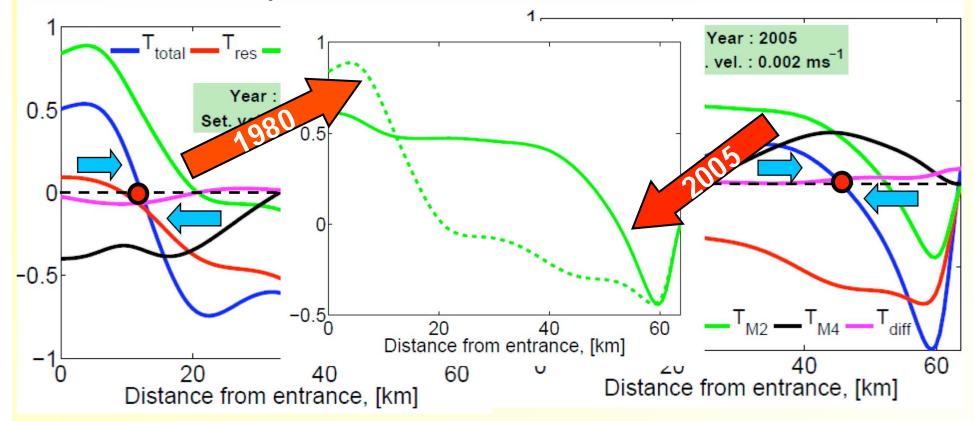
Maximum of sediment concentration coincides with zeros of transport function T.



The convergence point has moved upstream mainly due to change of the M₂ contribution

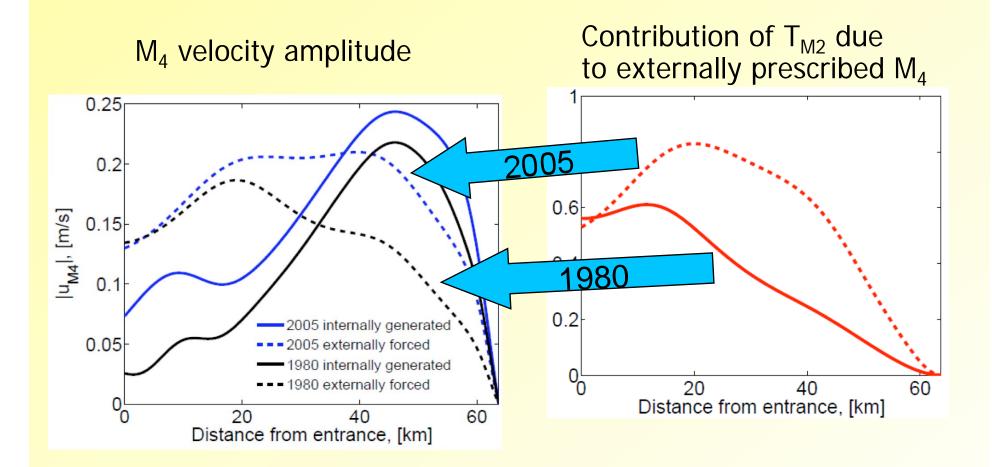
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Maximum of sediment concentration coincides with zeros of transport function T.



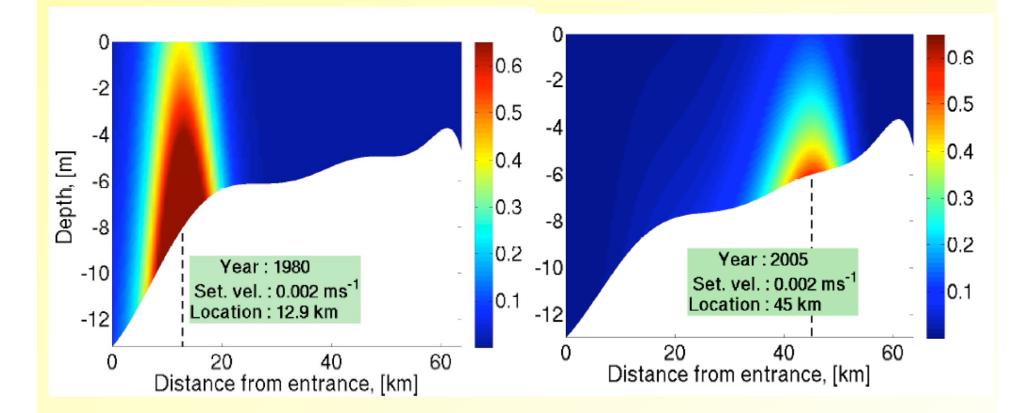
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Model Results (8)



The behavior of the T_{M2} component has changed due to changes in the externally prescribed M₄: <u>TIDAL ASYMMETRY MECHANISM</u>

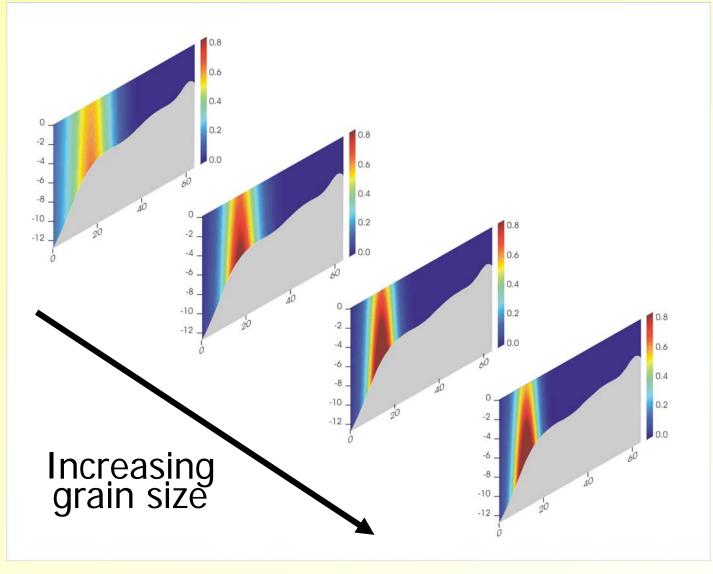
Model Results (9)



CHANGE OF TRAPPING LOCATION DUE TO CHANGE IN TIDAL ASYMMETRY (CHARACTER OF M4 TIDAL WAVE)

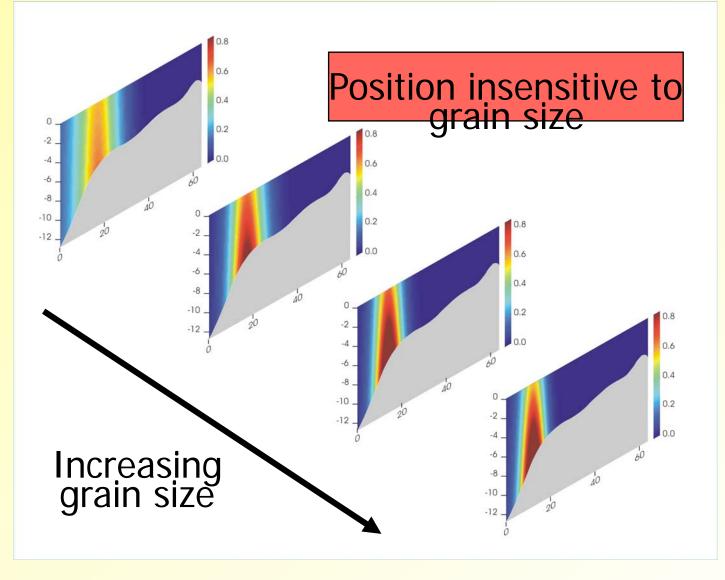
Model Results (10)

Dependency on grain size (1980)



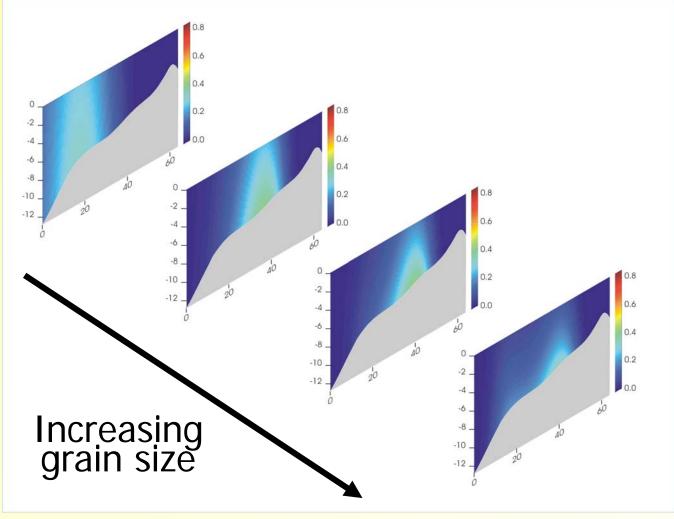
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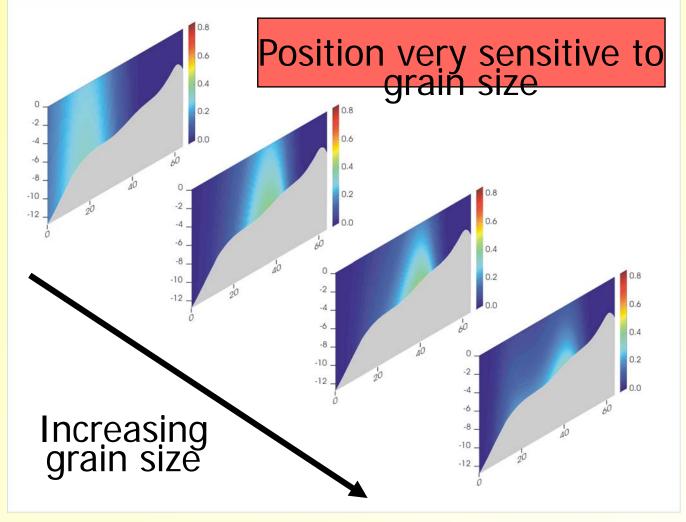
Model Results (11)

Dependency on grain size (2005)



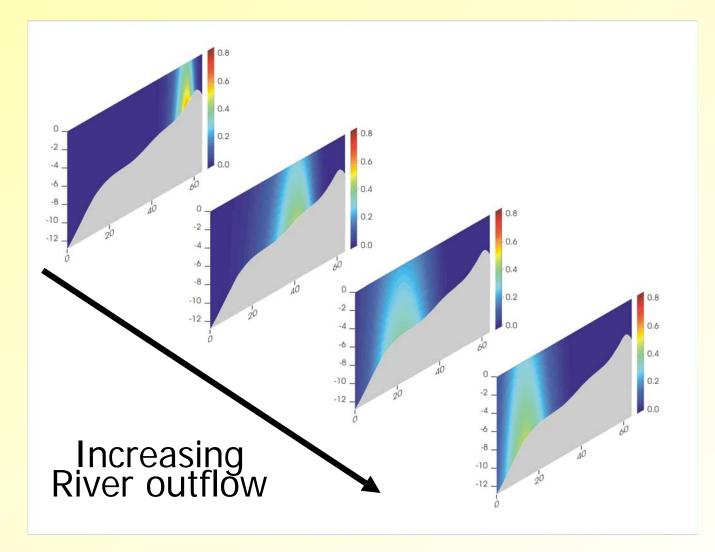
Model Results (11)

Dependency on grain size (2005)



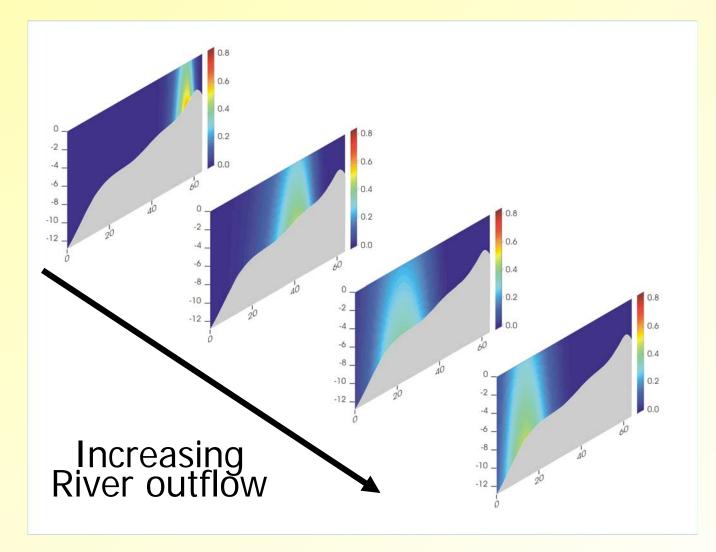
Model Results (12)

Dependency on river outflow (2005)



Model Results (13)

Dependency on river outflow (2005)



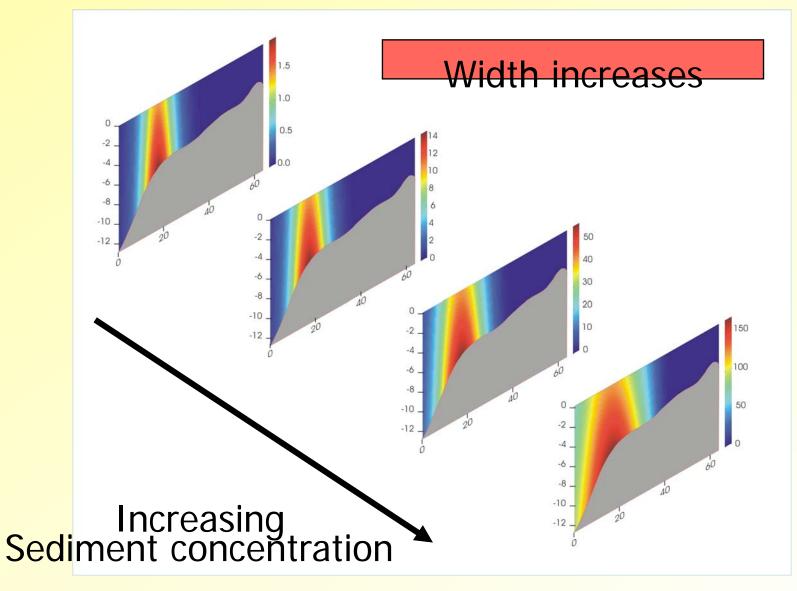
Model Results (14)

Extending the model by making density depend on Sediment concentration as well

Results in a nonlinear differential equation for the sediment availability a(x)

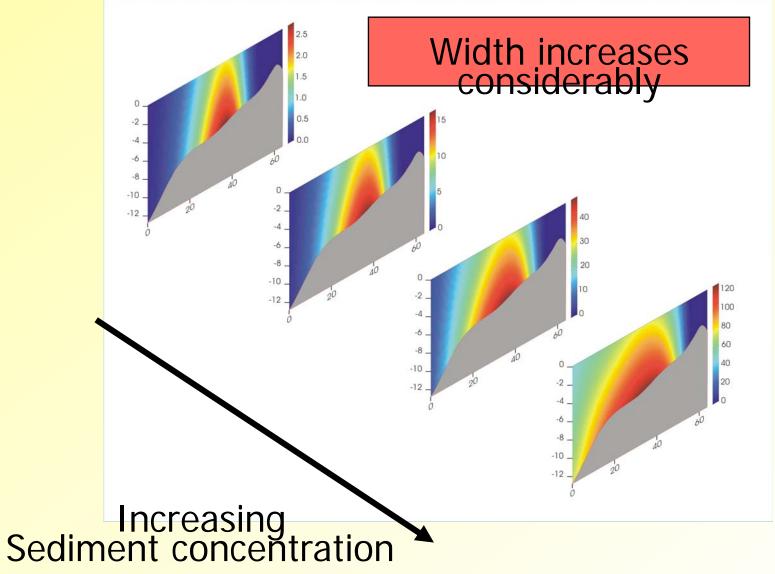
Model Results (15)

Results for the 1980 case



Model Results (16)

Results for the 2005 case



Conclusions

- Formation of ETMs can be modelled with an idealised model. Essential Ingredients:
 - Along-estuary varying erosion coefficient
 - (~ layer of fine sediment)
 - Tidal asymmetry
- Physical mechanisms resulting in the ETMs can be understood using the idealised model.
- Difference in trapping of sediment in Ems Estuary mainly a result of the changes in tidal asymmetry between 1980 and 2005.
- Model sensitivities to parameters and parameterizations can be easily investigated. The trapping locations in 1980 are not very sensitive to parameter changes, in 2005 the locations change dramatically when changing parameters.